# F4 FREEWAY MAYS HILL-PROSPECT 

## WORKING PAPER3-PROSPECT ARTERIAL

## PREPARED FOR :

THE DEPARTMENT MAIN ROADS

BY:
OVE ARUPTRANSPORTATION PLANNING
41 McLAREN STREET
NORTH SYDNEY 2060
2. TRAVEL SPEEDS 2
3. GREYSTANES ROAD

6
4. PROSPECT ARTERIAL 10
5. MODELLING ASSUMPTIONS 17
6. PREFERRED OPTION 19

T40:1.4161

## 1. INTRODUCTION

This working paper reviews the various points, questions and additional work raised with regard to the earlier working papers on the F4 (Mays Hill - Prospect).

Particular attention is paid to the traffic effects of the Prospect Arterial and desirability of an F4 interchange on Greystanes Road.

The modelling work for future traffic prediction has been carried out by the Strategic Planning section of the Department of Main Roads following discussions with the Consultants as to the various assumptions involved.

It is considered that the forecast flows are the best estimate of future traffic volumes that can currently be made, however it should be recognised that forecasting does entail a degree of uncertainty. This paper should be read in that light.

## 2. TRAVEL SPEEDS

To estimate travel speeds on the Great Western Highway, the Traffic Section of the Department of Main Roads conducted four surveys for different periods of the day in July 1985, November 1985 (two) and April 1985. A total of 118 runs was made during these surveys between Woodville Road and Parramatta and the F4 at Prospect and visa versa. Most of these runs (80) were done in April 1986 in order to get a more reliable indication of the travel speeds on shorter sections of the highway than in previous surveys.

A summary of the data collected during the surveys is shown in Tables 2.1 and 2.2, which represent the traffic under peak traffic flow conditions, i.e. : eastbound in the morning period and westbound in the evening peak period. It can be seen that the overall travel speeds per survey are within reasonable range limits. For eastbound traffic in the morning peak period they vary from $34 \mathrm{~km} / \mathrm{h}$ to $47 \mathrm{~km} / \mathrm{h}$, with an average of $37 \mathrm{~km} / \mathrm{h}$. The minimum and maximum overall travel speeds observed in any of the surveys during this period were $31 \mathrm{~km} / \mathrm{h}$ and $56 \mathrm{~km} / \mathrm{h}$ respectively. Per section, the travel speeds of eastbound traffic in the morning peak period varied from $12 \mathrm{~km} / \mathrm{h}$ between Church Street and Woodville Road at Parramatta to $93 \mathrm{~km} / \mathrm{h}$ between the F4 and Blacktown Road at Prospect. This is a wide range, which indicates strong fluctuations in travel speeds, due to traffic flows and signal coordination.

In the evening peak period, the overall travel speeds for westbound traffic are somewhat higher. They vary from $40 \mathrm{~km} / \mathrm{h}$ to 54 km/h in the April 1986 and the second November 1985 surveys respectively. The average overall travel speed is calculated at $42 \mathrm{~km} / \mathrm{h}$, with a minimum and maximum of $30 \mathrm{~km} / \mathrm{h}$ and $59 \mathrm{~km} / \mathrm{h}$ respectively. Per section the evening peak period travel speeds fluctuate from $8 \mathrm{~km} / \mathrm{h}$ between Greystanes and Toongabbie Roads to $89 \mathrm{~km} / \mathrm{h}$ between Blacktown and Reservoir Roads.

During the presentation of Working Paper 1 and 2 some doubt was expressed regarding the validity of the evening peak travel speeds, which were considered to be too high. The consultants carried out an additional survey on Tuesday, 8th July 1986 to check to results of previous surveys. This survey is summarised in column 6 of Table 2.2. It can be seen that this survey does show a somewhat lower overall travel speed, but this difference is not considered significant as the survey sample is rather small compared to the large fluctuations of the travel speeds.

It is considered that the doubt expressed in the presentation may be based on individual local driving perception. Low perceptions at travel speed may be produced by an individuals experience of congested conditions during a non average day. Low levels of service (or travel speeds) may be experienced on sections such as Church Street to the F4 Onload or Greystanes to Toongabbie Roads. The effects of these low travel speeds are generally perceived much more strongly than they are in reality, (for example in journey to work and modal split models, the value of waiting time is often twice the value of the actual travel time).

TABLE 2.1
TRAVEL SPEEDS ON GREAT WESTERN HIGHWAY

- EASTBOUND, MORNING PEAK PERIOD -

Cross Street Average Travel Speeds per Survey Period Min Max

| Sample Size | 5 | 5 | 5 | 10 |  |  |  |
| :--- | ---: | ---: | ---: | :--- | :--- | :--- | :--- |
| F4 - Prospect | 67 | 46 | 31 | 22 | 18 | 93 | 31 |
| to Reservoir Rd | 67 | 46 | 31 | 69 | 22 | 93 | 51 |
| to Blacktown Rd | 28 | 21 | 22 | 52 | 15 | 71 | 30 |
| to Toongabbie Rd 28 | 21 | 22 | 18 | 15 | 55 | 21 |  |
| to Greystanes Rd 63 | 53 | 45 | 30 | 16 | 74 | 40 |  |
| to Ettalong Rd | 63 | 53 | 45 | 27 | 15 | 74 | 38 |
| to Berith Rd | 63 | 53 | 45 | 44 | 31 | 82 | 49 |
| to Centenary Rd | 53 | 62 | 65 | 55 | 34 | 74 | 58 |
| to F4 On Load | 30 | 20 | - | 24 | 17 | 39 | 24 |
| to Church St | - | 33 | - | 19 | 12 | 52 | 22 |
| to Woodville Rd |  |  |  |  |  |  |  |
| Overall | 47 | 35 | 36 | 34 | 31 | 56 | 37 |

* Two independent surveys, which were conducted in the same week 85.1 represents data collected for a survey along Parramatta Road and the Great Western Highway. 85.2 represents data collected for a survey along the F4 Freeway, with missing links along the Highway and Parramatta Road.

TABLE 2.2
TRAVEL SPEEDS ON GREAT WESTERN HIGHWAY

- WESTBOUND, EVENING PEAK PERIOD -

| Cross Street |  |
| :---: | :---: |
|  | Average Travel Speeds per Survey Period (km/hr) Min Max Overal |
| July 85 Nov $85.1^{*}$ Nov. $85.2^{\star}$ Apr 86 July 86 ured ured aver age |  |


| Sample Size | 5 | 5 | 5 | 17 | 4 |  |  |  |
| :--- | ---: | ---: | ---: | ---: | ---: | ---: | ---: | :--- |
| Woodville Rd to | - | 36 | - | 43 | 29 | 20 | 61 | 39 |
| Church St to | 31 | 27 | - | 25 | 22 | 16 | 50 | 26 |
| F4 On Load to | 63 | 49 | 28 | 38 | 49 | 21 | 77 | 40 |
| Toongabbie Rd to 44 | 45 | 51 | 48 | 52 | 34 | 70 | 48 |  |
| Berith Rd to | 44 | 45 | 51 | 28 | 28 | 18 | 64 | 34 |
| Ettalong Rd to | 44 | 45 | 51 | 57 | 63 | 34 | 70 | 53 |
| Greystanes Rd to 42 | 41 | 40 | 23 | 17 | 8 | 50 | 27 |  |
| Toongabbie Rd to 42 | 41 | 40 | 41 | 58 | 25 | 71 | 42 |  |
| Blacktown Rd to | 37 | 69 | 73 | 64 | 44 | 23 | 89 | 57 |
| Reservoir Rd to | 37 | 69 | 73 | 48 | 57 | 23 | 87 | 51 |
| F4 - Prospect |  |  |  |  |  |  |  |  |
| Overall | 41 | 43 | 54 | 40 | 36 | 30 | 59 | 42 |

[^0]
## 3. GREYSTANES ROAD

### 3.1 Introduction

This section reviews the desirability of an intersection or overpass between Greystanes Road and the F4 Freeway.

### 3.2 Road Functions

The construction of a full intersection between Greystanes Road and the F4 Freeway would have the effect of reinforcing the traffic carrying function of Greystanes Road and would lead to longer distance regional traffic movements on a road designated as a sub-arterial in Traffic Authority and WSROC short and long term road hierarchies. The intersection would thus have the effect of creating a de facto arterial route.

With the construction of the Prospect Arterial, there would be a situation with three north-south arterial routes (Prospect, Greystanes and Jersey Road) within a distance of 4.7 km . It is considered that this is too close an arterial spacing within this semi-urban area and would not allow the development of a proper hierarchy of lower order roads within individual arterial 'boxes'.

The production of a Greystanes Arterial route would tend to reinforce use of Blacktown Road and Toongabbie Road and reinforce the volume and regional nature of traffic using these routes. Again the effect would be to force arterial functions on designated sub-arterial (and local) roads.

The creation of a Greystanes Arterial route would reduce the longer term need for the Prospect Arterial as employment and population would tend to locate to make use of the Greystanes Road.

Greystanes Road has not been designed as an arterial route. It has frequent intersections with local roads, residential frontage access, no medians etc. etc. Without considerable works to reduce the potential conflicts and enhance the capacity of Greystanes Road it would prove unsuitable for an arterial function and would be likely to have a high accident rate.

### 3.3 Environment

The additional traffic use of Greystanes and other feeder roads generated by construction of an intersection would have an effect upon the environment of those roads. Noise, pollution, vibration, accidents, visual intrusion etc. would all be worsened along Greystanes and other influenced roads. In addition the intersection itself would have an impact upon the environment of the local residential area.

Although in the shorter term the incremental environmental degradation may be difficult to perceive the longer term summation would produce significant environmental impact especially along Greystanes Road. As Greystanes Road has residential frontages it would be extremely difficult to introduce any measures to mitigate harmful environmental effects.

### 3.4 Traffic Effects

The construction of an F4 intersection on Greystanes is likely to have minor short term traffic effects on Greystanes Roads. However, in the longer term, if the Prospect Arterial is not constructed. It is estimated that the following flows would result on Greystanes Road during the morning peak period in 2011.

With F4 interchange (2B) and Prospect Arterial

With F4 crossover (2A) 1305 and Prospect Arterial

With F4 interchange (2B) 2755 and no Prospect Arterial

With F4 crossover (2A)
2095 and Prospect Arterial
(See Figures 3-14)

It may be observed that construction of an interchange increases likely flows by several hundred vehicles per hour. Non-construction of Prospect Arterial has a greater effect increasing likely flows by $700-1000$ vehicles per hour on Greystanes Road.

### 3.5 Physical Design

Although an F4 intersection on Greystanes Road can physically be constructed it would require land purchase and the loss of some local housing.
3.6 Timing

In the shorter term (the next few years) Jersey Road will supply some minor relief to Greystanes Road and Horsely Drive will act as a feeder to Jersey Road in approximately five years. However if the Prospect Arterial is not constructed for some time then traffic volumes will grow significantly on Greystanes Road. This growth being reinforced by the construction of an F4 intersection.

### 3.7 Conclusions

It is generally concluded that a full F4 intersection should not be constructed on Greystanes Road. Such an intersection would enforce an arterial role on Greystanes Road and thus produce functional, environmental and physical problems. Such an intersection would also delay the need for the Prospect Arterial and thus require capacity/safety improvements within Greystanes Road. As it is very unusual/unlikely that intersection ramps be demolished once installed it is considered that temporary construction of an intersection is not feasible.

## 4. PROSPECT ARTERIAL

The preferred freeway option is the parallel route; scenario 2 A describes a crossover between the F4 and Greystanes Road, scenario 2B includes a full intersection between the freeway and Greystanes Road.

Scenarios $2 A$ and $2 B$ were modelled with and without the Prospect arterial, with the aid of the DMR's TRANPLAN traffic assignment model, in order to gauge its effects on both networks. In particular the effects on the north-south routes and interchanges are described.

### 4.1 Effects on North-South Routes

The exclusion of the Prospect arterial tends to increase expected 2011 traffic volumes on the following north-south roads:
(i) Wallgrove;
(ii) Horsley;
(iii) Blacktown Road (between Church Lane and the Great Western Highway);
(vi) Jersey; and
(vii) Centenary.
and tends to decrease expected 2011 traffic volumes on Church Lane.

Figures 1 and 2 show the model predicted 2011 AM-peak hourly flows for scenarios $2 A$ and $2 B$ with and without the Prospect arterial.

The model predicted 2011 AM-peak 2 way volumes expected on these north-south roads are shown in Table 4.1 .1 below. The percentage of Prospect arterial traffic diverted to these roads is also indicated in brackets.

TABLE 4.1.1 MODEL PREDICTED VOLUMES ON N-S ROUTES

| Road | Model Predicted 2011 AM-Peak Volumes (2 way) for Scenario |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | 2A with Prospect Arterial | 2A without Prospect Arterial | 2B with Prospect Arterial | 2B without Prospect Arterial |
| Wallgrove Rd | 3245 | 4070 (24\%) | 3245 | 4070 (24\%) |
| Horsley Rd | 1650 | 2145 (15\%) | 1595 | 2145 (16\%) |
| Church Lane | 4400 | 2695 | 4345 | 2585 |
| Blacktown Rd | 770 | 1045 | 880 | 1210 |
| Greystanes Rd | 1320 | 2145 (24\%) | 1265 | 2090 (24\%) |
| Ettalong Rd | 825 | 990 (5\%) | 880 | 880 |
| Jersey Rd | 3025 | 3630 (18\%) | 3025 | 3575 (16\%) |
| Centenary Rd | 1210 | 1540 (10\%) | 1265 | 1540 (8\%) |
| Prospect arterial | 3410 | - | 3465 | - |

Note: Values in brackets represent the percentage of Prospect arterial traffic diverted to other north-south roads south of the Great Western Highway.

The above table illustrates the adverse effects which would occur on other north-south roads, particularly Greystanes, Jersey and Wallgrove Roads. It is evident that the Prospect arterial is an important north-south link which would alleviate the expected traffic increases on these other north-south connections, thereby benefiting the community in terms of distance saving, noise pollution and air pollution control.

The number of lanes required, on these north-south roads, to provide a peak hour level of service $C$ are shown in Table 4.1.2 below:
table 4.1.2 Number of Lanes required on n-S Routes

| Road | No. of Lanes (2 way) Required for Scenario |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
|  | 2A with Prospect Arterial | 2A without Prospect Arterial | 2B with Prospect Arterial | 2B without Prospect Arterial |
| Wallgrove Rd | 4 | 6 | 4 | 6 |
| Horsley Rd | 2 | 4 | 2 | 4 |
| Church Lane | 6 | 4 | 6 | 4 |
| Blacktown Rd | 2 | 4* | 2 | 4 |
| Greystanes Rd | 2 | 4 | 2 | 4 |
| Ettalong Rd | 2 | 2 | 2 | 2 |
| Jersey Rd | 4 | 6* | 4 | 6* |
| Centenary Rd | 2 | 2 | 2 | 2 |
| Prospect arterial | 6* | - | 6* | - |

Based on a level of service C and lane capacities provided by DMR Strategic Planning Section.

* Note predicted flows just exceed level of service 'C' capacity.

If the Prospect arterial is not built, traffic will be diverted to other north-south roads as shown in Table 4.1.1. Consequently, the following roads will require additional upgrading above that required in the preferred scheme:
(i) Wallgrove;
(ii) Horsley;
(iii) Blacktown (between Church Lane and the Great Western Highway);
(iv) Greystanes; and
(v) Jersey.

As a result, arterial functions will be forced on Greystanes and Blacktown Roads, which are designated sub-arterial roads in the Traffic Authority and WSROC short and long term road hierarchies.

### 4.2 Effect on Interchanges

The model predicted 2011 turning volumes were adjusted, based on professional judgment, in light of the surveyed 1986 turning volumes (conducted by the Consultants in April) along the Great Western Highway adjacent to the missing F4 freeway link. The adjusted turning volumes, predicted for the year 2011, are shown in Figures 3 to 14 inclusive.

The following table shows the figures which correspond to each scenario investigated.

SCENARIO

2A WITH PROSPECT ARTERIAL
2A WITHOUT PROSPECT ARTERIAL
2B WITH PROSPECT ARTERIAL
2B WITHOUT PROSPECT ARTERIAL

FIG. NO's

3 to 5 incl.
6 to 8 incl.
9 to 11 incl.
12 to 14 incl.

It should be noted that the general magnitude of turning volumes reflects the model predictions for the year 2011. These magnitudes may be reached earlier or later than expected as population/employment grows and the assumed distribution may also vary. These volumes thus reflect the best current estimate of 2011 flows but road design with considerable flexibility is desirable to facilitate future unforeseen changes in traffic patterns.

The exclusion of the Prospect arterial link affects the interchange ramp volumes as indicated in Tables 4.2.1 and 4.2.2 below:

TABLE 4.2.1:

| Interchange | Ramp | Predicted 2011 Interchange AM-Peak Ramp Volumes for Scenario 2A |  |
| :---: | :---: | :---: | :---: |
|  |  | With Prospect Arterial | Without Prospect Arterial |
| CHURCH LANE | Eastbound on-ramp | 990 | 1210 (+22\%) |
|  | Eastbound off-ramp | 565 | 375 (-34\%) |
|  | Westbound on-ramp | 610 | 570 (-7\%) |
|  | Westbound off-ramp | 660 | 715 (+8\%) |
| $\begin{aligned} & \text { JERSEY } \\ & \text { ROAD } \end{aligned}$ | Eastbound on-ramp | 880 | 890 (+1\%) |
|  | Eastbound off-ramp | 660 | 660 |
|  | Westbound on-ramp | 660 | 850 (+29\%) |
|  | Westbound off-ramp | 1155 | 1370 (+19\%) |

Note: The values in brackets represent the percentage increase/ decrease in traffic expected if the Prospect arterial is not built.

TABLE 4.2.2:

| Interchange | Ramp | Predicted 2011 Interchange AM-Peak Ramp Volumes for Scenario 2B |  |
| :---: | :---: | :---: | :---: |
|  |  | With Prospect Arterial | Without Prospect Arterial |
| CHURCH LANE | Eastbound on-ramp | 850 | 1210 (+42\%) |
|  | Eastbound off-ramp | 400 | 375 (-6\%) |
|  | Westbound on-ramp | 600 | 570 (-5\%) |
|  | Westbound off-ramp | 730 | 1125 (+54\%) |
| GREYSTANES ROAD | Eastbound on-ramp | 250 | 365 (+46\%) |
|  | Eastbound off-ramp | 565 | 520 (-8\%) |
|  | Westbound on-ramp | 165 | 315 (+91\%) |
|  | Westbound off-ramp | 325 | 530 (+63\%) |
| $\begin{aligned} & \text { JERSEY } \\ & \text { ROAD } \end{aligned}$ | Eastbound on-ramp | 700 | 870 (+24\%) |
|  | Eastbound off-ramp | 465 | 350 (-25\%) |
|  | Westbound on-ramp | 710 | 720 (+1\%) |
|  | Westbound off-ramp | 1000 | 1045 (+5\%) |

Note: The values in brackets represent the percentage increase/decrease in traffic expected if the Prospect arterial is not built.

The above tables illustrate the effect that the Prospect arterial has on the predicted 2011 AM peak ramp volumes. In general, the ramp volumes tend to increase if the Prospect arterial is not built.

The Prospect arterial has a greater effect on the north-south routes than on the interchange ramps as indicated in Section 4.1. The magnitude of this effect on other north-south routes reflects the need for the Prospect arterial. The Prospect arterial is considered an important component of the preferred scheme.

## 5. MODELLING ASSUMPTIONS

### 5.1 Introduction

In order to produce forecasts of traffic flows the Strategic Planning section of the DMR made use of the TRANPLAN model for Sydney following discussion with the Consultants as to the parameters involved. This section reviews the main modelling assumptions used.

### 5.2 The Network

The future network used was the year 2000 network with local changes for inclusion/exclusion of the F4 between Mays-Hill and Prospect and the Prospect Arterial.

The overall network included the following major new roads. A full list of roads and details of exact network descriptions are available from the strategic planning section of the DMR.

Major New Roads Included in Network

- Gore Hill Freeway
- Castlereagh Freeway (01d Windsor Rd - Pittwater Rd)
- Newcastle Freeway (Berowra to Pearces Corner)
- South Western Freeway
- Eastern Distributor
- Silverwater Arterial
- Johnston Creek Route
- Liverpool Arterial
- Horsley Drive
- Parramatta Ring Road.


### 5.3 Population/Employment Distribution

The population/employment distribution shown in the following table was prepared by the Department of Environment and Planning. The table suggests major growth in local centres particularly Baulkham Hills, Liverpool, Campbelltown and Blacktown. Parramatta is predicted to have a nett reduction in population but continued growth in employment.

It is considered that the population/emp loyment distribution used is the best available basis for future traffic modelling and is that used by the DMR for year 2000 strategic network planning.

| $\begin{aligned} & \text { LOCAL } \\ & \text { 6DVERTMENT } \\ & \text { AREA } \end{aligned}$ | population |  |  |  | Emplophent |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 1981 | 2011 | CHAHEE | $\%$ | 1981 | 2011 | CHange | $\%$ |
| 1 STomey | 51875 | 53686 | 1814 | 3.4 | 256259 | 256825 | 566 | 0.2 |
| 2 MOSTH SYOMEY | 48500 | 48392 | -108 | -0.2 | 44725 | 49489 | 4764 | 9.6 |
| 3 SOuTH SYONEY | 30776 | 31902 | 1126 | 3.5 | 48591 | 30343 | -18248 | -60.1 |
| 4 MOOLARRA | 51659 | 48739 | -292? | -6.0 | $14720^{-}$ | 14259 | -461 | -3.2 |
| 5 mhumerey | 61575 | 54682 | -6893 | -12.6 | 8304 | 17270 | 8966 | 51.9 |
| 6 RANOLICK | 116202 | 104211 | -11991 | -11.5 | 24691 | 33613 | 8922 | 26.5 |
| 7 EOTAKY | 35565 | 2965 | -5913 | -19.9 | 31326 | 32320 | 994 | 3.1 |
| 8 Marickulle | 83443 | 71145 | $-12303$ | -17.3 | 26367 | 20787 | -5580 | -26.8 |
| 9 LEICHAPT | 57293 | 51764 | -5509 | -10.6 | 22611 | 11480 | -11131 | -97.0 |
| 10 drungohe | 30961 | 29640 | -1321 | -4.5 | 6851 | 6637 | -214 | -3.2 |
| 11 ASHFIELS | 41253 | 37012 | -424i | -11.5 | 7365 | 8207 | 842 | 10.3 |
| 12 gunajob | 28896 | 32506 | 3612 | 11.1 | 9631 | 15183 | 5552 | 36.6 |
| 13 concoma | 23926 | 25654 | 1725 | 6.7 | 13545 | 12928 | -617 | -4.8 |
| 14 STRATHFIELO | 25882 | 30462 | 4580 | 15.0 | 16893 | 14028 | -2865 | -20.4 |
| 15 chaterumy | 126741 | 107836 | -18905 | -17.5 | 24994 | 24699 | -385 | -1.6 |
| 16 Rockale | 82857 | 76500 | $-6354$ | -8.3 | 18145 | 16961 | -1184 | -7.0 |
|  | 4632 | 51545 | 5223 | 10.1 | 10893 | 13023 | 2130 | 16.4 |
| 18 husstuille | 64910 | 64673 | -271 | -0.4 | 14466 | 22768 | 8302 | 36.5 |
| 19 SUTHEPLAM | 165336 | 205002 | 39726 | 19.4 | $3480{ }^{1}$ | 65331 | 30450 | 46.6 |
| 20 Emastohn | 1526.36 | 144674 | -7962 | -5.5 | 60841 | 59461 | -1380 | -2.3 |
| 21 dubur | 4662 | 50229 | 3607 | 7.2 | 34280 | 33774 | -501 | -1.5 |
| 22 hatrove | 80116 | 81495 | 1379 | 1.7 | 24715 | 33740 | 9025 | 26.7 |
| 23 fairfiel | 12955 | 16197 | 32364 | 20.0 | 25909 | 42781 | 16872 | 37.4 |
| 24 LIVEPPPOL | 92715 | 268653 | 175948 | 65.5 | 26941 | 92931 | 65990 | 71.0 |
| 25 Camiten | 17096 | 149973 | 132877 | 88.6 | 5435 | 36012 | 30577 | 84.9 |
| 2 CAHPBELLTOHE | 91525 | 176136 | - 84611 | 48.0 | 14192 | 79797 | 65665 | 82.2 |
| 27 PERPITH | 108720 | 216360 | 107640 | 49.8 | 26500 | 81487 | 55187 | 67.7 |
| 25 HINSSOR | 20870 | 50525 | - 29655 | 58.7 | 9323 | 15165 | 5842 | 38.5 |
| 29 BLACETOM | 181139 | 374193 | +193054 | 51.6 | 35124 | 120118 | 84994 | 70.8 |
| 30 Paprantita | 130443 | 129367 | -1556 | -1.2 | 62266 | 103035 | 40767 | 39.6 |
| 31 EadLhath hills | 97064 | 212815 | 119731 | 56.3 | 16884 | 59212 | 42328 | 71.5 |
| 32 honscey | 111081 | 127480 | 76399 | 12.9 | 24612 | 47350 | 22718 | 48.0 |
| 33 PYPE | 88948 | 85126 | -38\% | -4.5 | 32074 | 34928 | 2854 | 8.2 |
| 34 HUKTERS HILL | 12537 | 11465 | -1072 | -9.4 | 3255 | 2541 | -712 | -28.0) |
| 35 LAME COVE | 29113 | 28467 | -646 | -2.3 | 9335 | 10240 | 906 | 8.8 |
| 36 Hillougher | 52120 | 52570 | 459 | 0.9 | 34211 | 51348 | 17177 | 35.4 |
| 37 KU -RIN5-64i | 101051 | 102537 | 1585 | 1.5 | 16922 | 27957 | 110.55 | 39.5 |
| 38 harsingah | 172653 | 209823 | 37170 | 17.7 | 38827 | 67015 | 28188 | 42.1 |
| 39 maney | 37080 | 33269 | -3791 | -11.4 | 8118 | 1312 | 5005 | 38.1 |
| 40 MOSk觻 | 26200 | 27549 | 1344 | 4.9 | 5210 | 5995 | 785 | 13.1 |
| 41 hellondily | 5029 | 178984 | 177955 | 97.2 | 1224 | 43555 | 42331 | 97.2 |
| toial | 2954612 | 4028800 | 1073988 | 26.7 | 1151254 | 1727609 | 576355 | 33.4 |

## 6. PREFERRED OPTION

The preferred option is the full duplication of the Great Western Highway with interchanges at Church Lane and Jersey Road and overpasses at Greystanes, Ettalong and Centenary Roads. Use is made of the existing F4 freeway connection at Prospect and Mays Hill to provide an eastbound off-ramp and eastbound on-ramp respectively.

The preferred option is that scheme identified in Working Paper 2 (i.e. scenario 2A) with one modification - the exclusion of the westbound on ramp in the vicinity of Dog Kennel Road, Prospect.

The need for this on ramp is questionable as freeway access is suitably proposed at Church Lane and Wallgrove Road interchanges. It is assumed that the predicted 2011 AM-peak hourly volume using this on-ramp (i.e. approx. 280 vehicles) would continue westbound along the Great Western Highway and gain access to the freeway at the Wallgrove Road interchange.

### 6.1 Description of the Preferred Option

The preferred option is illustrated in Figure 15 and incorporates the following:
(i) A six lane freeway along the alignment as shown in Figure 15 between Prospect and Mays Hill. Six lanes are required in order to provide a level of service $C$ during peak hour periods for the 2011 forecast flows. Staged construction (four lanes with provision for six) may be appropriate for the Freeway.
(ii) Interchanges at Church Lane and Jersey Road. The number of ramp lanes required to provide a peak hour level of service $C$ are as follows:

| Interchange | Ramp | No. of Lanes Required |
| :--- | :---: | :---: |
| CHURCH LANE | Eastbound <br> on-ramp <br> Eastbound <br> off-ramp <br> Westbound <br> on-ramp <br> Westbound <br> off-ramp | 2 |


| Eastbound <br> on-ramp | 1 |
| :--- | :--- |
| Eastbound <br> off -ramp | 2 |
| Westbound <br> on-ramp | 2 |
| Westbound <br> off-ramp | 1 |

(iii) Partial retainment of the existing freeway connections with the Great Western Highway at both ends of the proposed freeway link. At both ends, it is recommended that the eastbound lanes be retained. The existing roadway will therefore provide a freeway off-ramp at the western end and an on-ramp at the eastern end. The number of lanes required to provide a peak hour level of service C are as follows:

| Off-Ramp | $1 *$ |
| :--- | :--- |
| On-Ramp | 2 |

* At present, two lanes are provided. It is therefore recommended that both are retained.


LEGEND

FORECAST INTERCHANGE
link volumes
FORECAST EASTBOUND AN WESTBOUND LINK VOLUMES

PROPOSED Closure

- blacktown rd
- proposed f4
freeway

MODEL FORECAST 2011 AM-PEAK HOURLY TRAFFIC VOLUMES (WITH PROSPECT ARTERIAL)


FULL DUPLICATION OF THE GREAT WESTERN HIGHWAY (SCENARIO 2A)


FULL DUPLICATION OF THE GREAT WESTERN HIGHWAY (SCENARIO 2B)

MODEL FORECAST 2011 AM-PEAK HOURLY TRAFFIC VOLUMES
WITHOUT PROSPECT ARTERIAL)

-

GREYSTANES
ROAD
$475 \longrightarrow$
$\longleftarrow 830$

## GREYSTANES ROAD OVERPASS

PREDICTED 2011 HOURLY VOLUMES FOR SCENARIO 2A WITH PROSPECT ARTERIAL




PREDICTED 2011 HOURLY VOLUMES FOR SCENARIO 2A WITH PROSPECT ARTERIAL


PREDICTED 2011 HOURLY VOLUMES FOR SCENARIO 2A WITHOUT PROSPECT ARTERIAL


GREYSTANES
$965 \longrightarrow$
$\leftarrow-1130$

## GREYSTANES ROAD OVERPASS



PREDICTED 2011 HOURLY


PREDICTED 2011 HOURLY VOLUMES FOR SCENARIO 2B WITH PROSPECT ARTERIAL



PREDICTED 2011 HOURLY VOLUMES FOR SCENARIO 2B WITH PROSPECT ARTERIAL


PREDICTED 2011 HOURLY VOLUMES FOR SCENARIO 2B WITHOUT PROSPECT ARTERIAL



PREDICTED 2011 HOURLY VOLUMES FOR SCENARIO 2B WITHOUT PROSPECT ARTERIAL



[^0]:    * Two independent surveys, which were conducted in the same week 85.1 represents data collected for a survey along Parramatta Road and the Great Western Highway. 85.2 represents data collected for a survey along the F4 Freeway, with missing links along the Highway and Parramatta Road.

